Study of Bond Strength of Plain Surface Wave Type Configuration Rebars with Concrete: A Comparative Study

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ABSTRACT

This study investigates the bond strength behaviour of plain surface wave type configuration (PSWC) rebars in comparison to mild steel (MS) and high yield strength deformed (HYSD) rebars of varied rib configuration as per BIS and ASTM standards. The variables in the rebar include plain surface, curved surface, parallel rib, diamond rib and Nano modified cement polymer anticorrosive coating (CPAC). Total of 30 pull-out specimens and 12 beam-end specimens were put to a pull-out test following BIS and ASTM standard respectively. The load corresponding to 0.025mm free end (FE) slip and 0.25mm loaded end (LE) slip were carefully observed. The load-deflection behaviour, appearance of the first crack in the specimens and ultimate failure load was recorded. The experimental results showed that as compared to MS rebars, HYSD rebars offer an approximately threefold increase in ultimate bond strength and 1.5 times increase in usable bond strength irrespective of varied rib configuration. PSWC rebars with 4mm offset and 80mm pitch offered 2.4 times increase in ultimate strength and 76.2% increase in usable bond strength as compared to MS rebars. The ultimate pull-out load of PSWC rebars was around 25% and the usable bond strength was only 8.6% lesser than HYSD rebars with parallel ribs. The adopted coating enhanced the corrosion resistance and the reduction in bond strength with any surface configuration was less than the permissible maximum reduction of 20% as specified in IS 13620-1993. Hence it can be concluded that PSWC rebars offered promising bond strength results and upon further optimization and study in other aspects, PSWC rebars can be a way to replace HYSD rebars in future for enhancing concrete durability at zero added cost.

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1. INTRODUCTION

The durability problem of concrete structures reinforced with HYSD rebars is worldwide resulting in early age failures and renovation costs add a large amount in annual expenditures [1, 2]. Neville [3] suggests reasons as “poor understanding of deterioration processes, inadequate acceptance criteria of concrete at site, and changes in cement properties and construction practices”. The major prominent threat unquestionably is corrosion of reinforcing steel, causing cracking, staining, and spalling of the cover of RC elements [4, 5]. This can result in unserviceable structures which can be unsafe for the occupants. Alekseev, et al. [6] commented on the above scenario as “the durability of reinforcement specimens with a stepped (deformed) profile may be roughly an order less than that of smooth specimens since the former have stress concentrators on the surface at the bases of projections, which represent sites of preferential formation of cracks”.

Anil [7] reported the yield strength as well as the bond strength of HYSD rebars is higher in comparisons to plain round MS rebars and concluded that there are certain durability issues concerning HYSD rebars in reinforced concrete structures like problems of early distress and associated failures of reinforced concrete structures built using HYSD rebars due to early corrosion. The observations by CPWD [8], Swamy [9], and Papadakis, et al. [10] are evidence of old concrete structures which were reinforced with MS rebars, performing much better than more recent structures reinforced with ribbed CTD and TMT rebars when such structures were subjected to the same environment.

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To attain a substitute and economical solution for overcoming the early corrosion problem in using HYSD rebars in reinforced concrete structures, an innovative type of reinforcing steel rebar named as PSWC rebar with a normal plain round surface having slightly curved axis has been proposed [7]. The tension (excursion from the original straight axis) is merely 4-8 millimetres as shown in Figure 1.

The PSWC rebar having offset-length of 4mm was selected for the study. The selection of the parameters was done based on the literature study [11-14].

In plain rebars, the ultimate pull-out force is not unlike as the load at which initial noticeable slip occurs, but in ribbed rebars, the ultimate pull-out load may resemble a greater slip which may not be obtained practically before other major failures occur. Thus in the study, the ultimate pull-out/failure load and complete load-slip behaviour of the selected rebars was observed and compared.

2. SCOPE AND OBJECTIVE OF THE STUDY

The strength aspect of PSWC rebar has to be tested to prove its viability of replacing the conventional rebars in concrete structures. Hence the bond strength of PSWC rebars in comparison with MS and HYSD rebar with varied rib configuration was presented in the study. Also, the influence of Nano modified CPAC on bond strength development has been included and compared following BIS guidelines.

3. MATERIAL PROPERTIES & MIX DESIGN

3.1. Concrete Mix Proportioning

The M30 concrete mix was formulated as per IS 10262-2009 [15], “Ordinary Portland Cement, 53-grade approved by IS 12269-1987” [16], fine aggregate (FA) of zone II as specified in IS 383-1970 [17] and 20 mm downgraded blue granite coarse aggregate (CA) was used. The proportioning of ingredients per m³ of concrete are presented in Table 1 with w/c ratio obtained as 0.45.

3.2. Reinforcing Rebars

To maintain quality throughout the study samples of selected 16mm diameter of MS of Fe250 grade, HYSD parallel ribs and HYSD diamond ribs rebars of Fe500 grade conforming to IS

<table>
<thead>
<tr>
<th>TABLE 1. Mix proportion</th>
</tr>
</thead>
<tbody>
<tr>
<td>Cement (kg/m³)</td>
</tr>
<tr>
<td>438</td>
</tr>
<tr>
<td>Mix ratio:</td>
</tr>
</tbody>
</table>

1786-2008 [18] were tested to study the mechanical properties and chemical composition. The tension test outcomes as per IS 1608:2005 [19] are stated in Table 2.

Table 3 includes the chemical composition of rebars used in pullout tests. The tests were conducted in ‘Chennai Mettex Laboratory’. The test results were compared with standard values set by major steel-producing industries and other premier research centres. The outcomes were found in the optimum range confirming the use of quality steel in the study.

3.3. Development of Cement Polymer Anti-corrosive Coating System

The Nano modified CPAC on the rebars was applied following IS 13620-1993 [20] guidelines. The “site oriented CPAC (passivating type) was composed of nitrite, styrene-butadiene polymer and other additives” [21]. The polymer solution was milky white, basic pH of 12.5 and a density of 1.035g/cm³. This anticorrosive polymer solution was compatible with concrete or cement paste.
when uniformly mixed with fresh OPC. The procedure involved the removal of loose rust and scales from the steel rebars by hard wire brush before brush coating [21]. The Nano modification in the CPAC was done by incorporating 5 gram of Nano Titanium Dioxide (Nano TiO$_2$) in 1 litre of CPAC. The thickness of the coating ranges from 150±25μm for 1 coat and 225±25μm for 2 coat measured by pull-off type thickness gauge. The treatment duration was 12 hours.

4. EXPERIMENTAL PROGRAM

The Universal Testing Machine (UTM) of capacity 1000 kN and capable of load increment at the rate of 2250 kg/minute was used for testing. The load cell of 500 kN (Model: ELC-30S) was used in the test setup. Dial micrometres were used at both FE and LE of the rebars to measure corresponding slip. A 20 mm rebar length from the rear face of the concrete specimen was provided with proper facing done to measure the FE slip and also the sufficient rebar from the front face was provided to safeguard the rebar in the UTM. “Polyvinyl Chloride (PVC) pipes were used as a bond breaker to restrict the bonded length of the rebars and to avoid a localized cone-type of failure of concrete at the LE of the specimen” [22, 23]. The standard procedure followed was as per IS 2770 (Part 1)-1967 [23] and ASTM A944-10 [24]. The minimum load corresponding to 0.025 mm FE and 0.25 mm LE slip was considered for calculating the usable bond strength throughout the study. Equation (1) is recommended to calculate bond stresses.

$$u = \frac{F}{\pi d_r l_r}$$  

where $F$ is the force in rebar, $d_r$ is the diameter and $l_r$ is the bond length of the rebar.

4.1. BIS Pull-out Specimens

Pullout specimens of dimensions ‘150×150×150 mm’ were cast with centrally embedded test rebar. At the FE, dial micrometre with least count of 2.5×10$^{-3}$ mm with a range of 2.5 mm was used. At the LE, dial micrometre with least count of 2.5×10$^{-2}$ mm and a range of 12.5 mm was used. The bonded length was restricted to 80 mm in all the test rebars. The mould, mixing and curing of specimens conform to the requirements as specified in IS 516-1959 [25]. In the LE, the concrete cube was placed on a bearing arrangement of similar dimensions with 18 mm hole in the centre to accommodate the test rebar. A helical of 6 mm diameter, MS rebar conforming to Grade I of IS 432 (Part 1)-1982 at 25 mm pitch [23] was provided as reinforcement.

Totally 30 BIS pull-out specimens were cast and tested. Figure 2 shows the different types of rebars that were tested for bond strength as per the procedure.

Figure 3 shows the reinforcement and arrangement of mould for casting pull-out specimens.

After 28 days of curing a thin and neat layer of good strength, gypsum plaster was applied on the specimens before 2 hours of testing to assure proper seating of the specimens in the test setup. Figure 4 shows the view of casted BIS pull-out specimens and Figure 5 illustrates the pull-out test setup.
4.2. ASTM Beam End Specimens  
As per ASTM A944-10 [24], the specimen shall consist of the test rebar cast in a block of RC with dimensions as follows:

i. \[600 + 25\text{mm}\] length

ii. \[(\text{db}+200+13\text{mm})\] width

iii. Minimum \[(\text{db}+\text{cb}+\text{le}+60\text{mm})\] height

Notations:

\ \text{cb} =\ \text{concrete cover in mm}
\ \text{db} =\ \text{nominal diameter of test rebar in mm}
\ \text{le} =\ \text{embedding length in mm}

Four stirrups were provided on the two flexural reinforcing rebars on either side of the test rebar and placed inline to the length of a specimen. Figure 6 shows the reinforcement and arrangement of mould for casting beam-end specimens.

4.2.1. Modifications Done in Beam End Specimens

The specimen was scaled down to suit the testing facility. The beam-end specimens were scaled down to 75\% of the recommended size that is 25\% of the length was reduced. To the scaled-down length of the specimen, the reinforcement was also scaled down.

Table 4 shows the details of the original and scaled-down specimen.

The bonded length was restricted to 200mm in all the test rebars. PSWC rebar of 4mm offset and 200mm pitch length was used to compare with conventional rebars. The flexural reinforcement having 0.5 times the cross-sectional area of the test rebar was provided with 4 rings of size ‘200mm×110mm’ as side face reinforcement in which each flexural rebar was provided with 2 rings. Figure 7 shows the PSWC rebar of 4mm offset and 200mm pitch length.

Subsequent 28 days of curing, the specimens were tested in the UTM with fabricated testing apparatus. Two dial gauges of accuracy 0.001 mm and 0.01 mm were used to measure the slip of the rebars at the FE and LE. Figure 8 shows the casted beam-end specimens and Figure 9 shows the test setup.

5. RESULTS AND DISCUSSION

5.1. Summary of BIS Pull-out Test Results

Table 5 shows the test observation in BIS pull-out specimens. The variation in the usable bond strength has
been calculated with respect to MS uncoated rebar. The coating thickness mention was a mean of a minimum of five readings that were taken throughout the length of rebar.

Figure 8. Casted beam-end specimens

Figure 10 shows the modes of failure in BIS pull-out specimens. From left to right it represents yielding of steel, pullout failure and pullout associated with splitting of concrete.

Table 5. Observations on pullout test

<table>
<thead>
<tr>
<th>No.</th>
<th>Type of Rebar</th>
<th>Load (kN)</th>
<th>Usable Bond Strength (MPa)</th>
<th>Mean Variations (%)</th>
<th>Coating Thickness (µm)</th>
<th>Mean cube compressive strength of concrete (MPa)</th>
<th>Ultimate Pullout Load (kN)</th>
<th>Average Ultimate Pullout Load (MPa)</th>
<th>Variation (%)</th>
<th>Type of Failure</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>S1MS UR</td>
<td>17.91</td>
<td>16.89</td>
<td>4.20</td>
<td>-</td>
<td>-</td>
<td>32</td>
<td>-</td>
<td>Symmetry</td>
<td>Pullout</td>
</tr>
<tr>
<td>2</td>
<td>S2MS UR</td>
<td>20.71</td>
<td>18.72</td>
<td>4.65</td>
<td>-</td>
<td>-</td>
<td>34</td>
<td>32</td>
<td>Symmetry</td>
<td>Pullout</td>
</tr>
<tr>
<td>3</td>
<td>S3MS UR</td>
<td>15.11</td>
<td>15.06</td>
<td>3.75</td>
<td>-</td>
<td>-</td>
<td>30</td>
<td>-</td>
<td>Symmetry</td>
<td>Pullout</td>
</tr>
<tr>
<td>4</td>
<td>S1HYSD PR UR</td>
<td>37.80</td>
<td>32.60</td>
<td>8.10</td>
<td>-</td>
<td>-</td>
<td>102</td>
<td>-</td>
<td>Symmetry</td>
<td>Pullout</td>
</tr>
<tr>
<td>5</td>
<td>S2HYSD PR UR</td>
<td>42.67</td>
<td>33.46</td>
<td>8.32</td>
<td>+92.86%</td>
<td>31.47</td>
<td>105.7</td>
<td>102</td>
<td>+218.75%</td>
<td>Splitting</td>
</tr>
<tr>
<td>6</td>
<td>S3HYSD PR UR</td>
<td>32.93</td>
<td>31.74</td>
<td>7.89</td>
<td>-</td>
<td>-</td>
<td>98.3</td>
<td>-</td>
<td>Symmetry</td>
<td>Splitting</td>
</tr>
<tr>
<td>7</td>
<td>S1HYSD DR UR</td>
<td>39.00</td>
<td>35.37</td>
<td>8.80</td>
<td>-</td>
<td>-</td>
<td>112.61</td>
<td>-</td>
<td>Symmetry</td>
<td>Yield</td>
</tr>
<tr>
<td>8</td>
<td>S2HYSD DR UR</td>
<td>44.87</td>
<td>38.07</td>
<td>9.47</td>
<td>+109.52%</td>
<td>29.66</td>
<td>115.77</td>
<td>112.61</td>
<td>+251.90%</td>
<td>Yield</td>
</tr>
<tr>
<td>9</td>
<td>S3HYSD DR UR</td>
<td>33.13</td>
<td>32.67</td>
<td>8.12</td>
<td>-</td>
<td>-</td>
<td>109.45</td>
<td>-</td>
<td>Symmetry</td>
<td>Yield</td>
</tr>
<tr>
<td>10</td>
<td>S1HYSD PR 1C</td>
<td>40.06</td>
<td>25</td>
<td>6.21</td>
<td>150.5</td>
<td>97.60</td>
<td>-</td>
<td>-</td>
<td>Symmetry</td>
<td>Splitting</td>
</tr>
<tr>
<td>11</td>
<td>S2HYSD PR 1C</td>
<td>34.96</td>
<td>23</td>
<td>5.71</td>
<td>+47.86%</td>
<td>151</td>
<td>101.6</td>
<td>97.60</td>
<td>+205%</td>
<td>Splitting</td>
</tr>
<tr>
<td>12</td>
<td>S3HYSD PR 1C</td>
<td>45.16</td>
<td>27</td>
<td>6.71</td>
<td>150</td>
<td>93.6</td>
<td>-</td>
<td>-</td>
<td>Symmetry</td>
<td>Splitting</td>
</tr>
<tr>
<td>13</td>
<td>S1HYSD PR 2C</td>
<td>24.2</td>
<td>25</td>
<td>6.01</td>
<td>250</td>
<td>90.20</td>
<td>-</td>
<td>-</td>
<td>Symmetry</td>
<td>Splitting</td>
</tr>
<tr>
<td>14</td>
<td>S2HYSD PR 2C</td>
<td>23.7</td>
<td>21.91</td>
<td>5.45</td>
<td>+39.52%</td>
<td>31.65</td>
<td>84.78</td>
<td>-</td>
<td>Symmetry</td>
<td>Splitting</td>
</tr>
</tbody>
</table>
The following observations were noted in the load-slip behaviour of rebars in BIS pull-out specimens:

(a) From the load-slip behaviour of MS UR, HYSD PR UR and HYSD DR UR revealed that at 0.025mm FE slip value, the load values observed for HYSD PR were in line with HYSD DR configuration. However, the MS R shows some initial resistance to slip with load increment but once the initial slip in the rebar occurs there was further huge slip observed on an increment of load in both FE and LE. The average ultimate load in MS UR was observed as 32kN with a usable bond strength of 4.20MPa. Similarly, the ultimate load for HYSD PR UR and HYSD DR UR was observed as 102kN and 112.61kN respectively. The usable bond strength value of HYSD PR UR and HYSD DR UR was observed as 8.10MPa and 8.80MPa respectively.

(b) From the load-slip behaviour of HYSD PR UR, HYSD PR 1C and HYSD PR 2C revealed that the peak load sustained by one and two coated rebars was 97.60kN and 90.20kN respectively. However, for uncoated rebar...
the ultimate pull-out load was 102kN. The single coated rebars carried 4.31% and two coated rebars carried 11.57% lesser load than uncoated rebars. Usable bond strength of single coated rebars was 23.33% and for double-coated rebars was 27.65% lesser than uncoated rebars.

(c) From the load-slip behaviour of HYSD DR UR, HYSD DR 1C and HYSD DR 2C. It was observed that single and double coated rebar withstands 6.47% and 10.61% lesser ultimate load respectively than uncoated rebar. Usable bond strength of single coated rebars was 23.18% and for double-coated rebar was 29.55% lesser than uncoated rebars.

(d) From the load-slip behaviour of MS UR, MS R 1C and MS R 2C. The ultimate load-carrying capacity of single and double-coated rebars was less by 18.75% and 21.88% respectively when compared to uncoated rebars. The usable bond strength for single coated rebars was 4.76% and for double-coated rebars was 11.90% lesser when compared to uncoated rebars.

(e) From the load-slip behaviour of MS UR, HYSD PR UR and PSWC UR with 4 mm deformation and 80 mm pitch length. It was observed that MS rebar carries 138.75% lesser load than PSWC rebar. HYSD PR UR showed 33.50% greater ultimate load carrying capacity and usable bond strength of just 9.46% greater than PSWC rebars.

5. 1. 1. Evaluation of Initial Crack and Ultimate Load of MS Rebars, HYSD Rebars and PSWC Rebars Figure 11 shows the evaluation of loads at which the first crack was visible and the ultimate load at which specimens failed in the pull-out test. PSWC rebars perform better than MS rebars. In HYSD rebar with parallel and diamond rib configuration, there was an appreciable difference between the first visible crack load and ultimate load. The corresponding difference between load at which first visible crack in HYSD PR UR and PSWC rebar was comparatively low.

5. 2. Summary of ASTM Beam-end Specimens Table 6 shows the observation of the pull-out test in beam-end specimens. The variation in usable bond strength has been calculated with respect to MS uncoated rebar. Figure 12 shows the crack pattern observed in the specimens during the test.

![Figure 11. Evaluation of initial crack load and ultimate load](image1)

![Figure 12. Crack pattern in beam-end specimens](image2)
The following observations were noted in the load-slip behaviour of beam-end specimens:

(a) From the load-slip behaviour of MS UR, HYSD PR UR and HYSD DR UR. The average ultimate load-carrying capacity of HYSD PR UR and HYSD DR UR was 219.32 and 236.13% greater than MS UR respectively. The usable bond strength of HYSD PR UR and HYSD DR UR was 57.37 and 68.85% greater than MS UR respectively.

(b) From the load-slip behaviour of MS rebar and PSWC rebar with 4mm profile deformation and 200mm pitch length, the PSWC rebar offered significantly improved resistance against slip in the initial stage as compared to MS rebar. PSWC rebar offered appreciably higher ultimate bond strength, 119.60% greater than MS rebar due to the presence of offset (ridge) and pitch (valley) of the steel-concrete interface. PSWC rebar showed significantly higher usable bond strength of the order of 48.0% greater as compared to MS rebar.

5. 2. 1. Evaluation of Initial Crack Load and Ultimate Load of MS Rebars, HYSD Rebars and PSWC Rebars

Figure 13 shows the evaluation of loads at which the first crack was visible in the specimen and the ultimate load at which the specimen fails in the pull-out test. It was evident that PSWC rebars perform better than MS rebar in the pull-out test. In HYSD rebar with parallel and diamond ribs, there is an appreciable difference between first crack load and ultimate load. The corresponding difference between the load at which the first crack is visible in HYSD PR UR and PSWC rebar was less.

Figure 14 shows the embedded coated rebars and the concrete at the end of the test. It was observed that the...
uncoated rebars were corroded more and the coating is more adhesive to the concrete than rebar. Figure 15 shows the scanning electron microscope (SEM) images of Nano modified CPAC adopted in the study.

6. CONCLUSIONS

The experimental and comparative study on bond strength of plain surface wave type configuration rebars (PSWC) with concrete was carried out as per BIS and ASTM procedure. In addition to this, a Nano TiO_{2} altered cement polymer anti-corrosive coating (CPAC) was included in the study to access its mechanical and durability properties. The following conclusions were noted.

(a) Irrespective of surface configuration, the bond strength of uncoated rebars was found more than that of coated rebars.

(b) There is a marginal rise in usable bond strength and the peak pull-out load of HYSD DR rebars as compared to HYSD PR rebars by 5.11%.

(c) As compared to MS rebars, HYSD rebars offered an approximately threefold increase in ultimate bond strength and 1.5 times increase in usable bond strength irrespective of rib configuration. The ultimate load-carrying capacity of coated HYSD diamond rib rebars surpassed mild steel rebars by four times in few cases.

(d) In BIS pull out test, PSWC rebars with 4mm offset and 80mm pitch offered ultimate load-carrying capacity of 76.40kN that is 2.4 times more than MS rebars. Also, there was a rise in usable bond strength by 76.20% compared to MS rebars.

(e) In ASTM beam-end specimens, PSWC rebars with 4mm offset and 200mm pitch offered ultimate load-carrying capacity of 78.4kN that is 2.2 times more than MS rebars. Also, there was a rise in usable bond strength by 48.36% compared to MS rebars.

(f) PSWC rebars exhibit an improved slip resistance and well-established load-slip behaviour as compared to MS rebars.

(g) In BIS pull-out test the ultimate bond strength of PSWC rebars was around 33.5% less as compared to uncoated HYSD rebars and the usable bond strength was about 9.5% less than for HYSD rebars with parallel ribs.

(h) The reduction in bond strength of coated rebars with any rib configuration was less than the maximum reduction of 20% specified by IS 13620-1993. Both 1coat and 2coated rebars satisfied IS code provisions.

(i) PSWC rebars with 4mm offset and 80mm pitch offered promising bond strength. Upon further optimization and testing of the rebar in other aspects, PSWC rebar can be future rebar to replace HYSD rebars for durable concrete construction at zero additional cost.

7. REFERENCES


 Persian Abstract

چکیده

این مطالعه رفتار استحکام پیکربندی میلگردهایی با روش خوشه‌کننده (PSWC) را در مقایسه با میلگردهای فولاد خیف (MS) و تغییر شکل مقاومت عامل‌کرده‌های با استاندارد (HYSUD) با سایر مدل‌های ممکن بررسی می‌شود. برای مقایسه استحکام میلگردهای مفید با استاندارد ASTM و BIS، پوشش خوشه‌کننده پیکربندی سیمان اصلاح شده و استحکام باند میلگردهای MS و HYSUD در آزمون‌های کشش و شکست ثبت شد. نتایج تحقیق نشان داد که استحکام پیکربندی میلگردهای MS از MS به سبب افزایش مقاومت به حدود 24 درصد زیادتر است. در مقایسه با MS، میلگردهای HYSUD با استاندارد ASTM به سبب افزایش مقاومت به حدود 20 درصد زیادتر است. در مقایسه با MS، میلگردهای HYSUD با استاندارد ASTM به سبب افزایش مقاومت به حدود 20 درصد زیادتر است.